



**REPORT OF PILE LOAD TEST
FOR PILES UNDER STATIC AXIAL
COMPRESSIVE LOAD**

**FOR THE
SAENGER THEATER REDEVELOPMENT
1101 CANAL STREET
NEW ORLEANS, LOUISIANA**

**TESTED FOR
BROADMOOR L.L.C.
2740 NORTH ARNOULT ROAD
METAIRIE, LOUISIANA**

ATTN: MR. SKENDER KEK

REPORT NO: C12-010-40

JULY 27, 2012

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PROJECT INFORMATION

Stratum Engineering, LLC (SE) was authorized to perform one (1) pile load test for the Saenger Theater Redevelopment on Canal Street and Basin Street in New Orleans Louisiana. Authorization to perform the pile load test was provided through a contract between Broadmoor and SE to provide testing and inspection services for the project. The work was performed in accordance with our Proposal C11-043R, dated September 14, 2011.

SCOPE OF WORK

The scope of work included monitoring the driving of one (1) large timber test pile and loading the pile to 50 tons or failure, whichever occurs first, in accordance with ASTM D1143 standard procedure. The purpose of the load test was to verify the pile design capacity of 12 tons. In addition, the scope included vibration monitoring during pile driving to collect data for others to establish threshold level for vibration during construction.

LOAD TEST PROCEDURE

One (1) static axial compressive load test was performed on a large timber test pile TP-1 driven at a non-permanent location near the building addition footprint to a depth of about 41 feet below the existing ground surface. Since the building piles will be tipped in the sand with a pile cut off elevation about 13 feet below the street level, the pile was driven within an 18 inch diameter steel casing reamed out to a depth of 13 feet to disengage friction contributed by the upper 13 feet of the pile and to simulate the loading condition the pile will undergo. The pile was load tested after a waiting period of 14 days. The load was applied to the test pile with a calibrated hydraulic jack and load cell assembly placed on the test pile and jacked against a steel beam extending over the top of the test pile. The steel beam was secured in place by a reaction frame anchored by 2 H piles installed to a depth of 65 feet. The load was applied to the test pile in one and one half (1.5) ton increments up to 50 tons in general accordance with ASTM D1143, "The Quick Load Method". The load was then released in four (4) equal decrements and the settlement after rebound was monitored for 30 minutes.

Settlement of the pile was determined by securing readings with an engineer's level trained on scales calibrated in 100ths of an inch. A second measuring system consisted of a piano wire, a mirror and a scale calibrated to 100ths of an inch set on one side of the pile conforming to the requirements of the governing standard. Benchmarks were also established for the test pile, which were verified each time the pile settlement was checked. Records of the pile settlement are included in subsequent sections of the report. The pile load versus settlement curve is included in the appendix.

VIBRATION MONITORING

Vibration was monitored during installation of the piles. Pile driving induced vibration was measured using mini-seis seismograph, Serial #4866. The monitor was set near the adjacent building at the corner of Rampart and Iberville Street. A peak particle velocity ranging between 0.045 to 0.085 inch per second (ips) was recorded during the driving of the test pile and the reaction piles which were located about 50 to 80 feet from the monitor. A peak particle velocity of 0.44 ips was recorded while the crane was moving the wooden mats across the site. Detailed records of vibrations are included in the appendix.

PILE DRIVING DATA

Three (3) piles including the test pile and two (2) reaction piles were driven by MR Pittman Group, L.L.C. The test pile was inspected at the site prior to driving. The test pile was driven near the proposed building addition at a non-permanent location. The test pile was driven on July 11, 2012 using a Vulcan 01 air activated hammer developing 15,000 ft-lbs. of energy per blow. The H piles were driven using a vibratory hammer (MKT Model V-20) to a depth of 41 feet then advanced to a penetration depth of 65 feet using a Vulcan 01 air activated hammer. The reaction piles were not monitored by SE. Therefore, the dimensions and penetration resistance of the test pile only are tabulated below:

File No.	TP-1
Tip Diameter, in.	7
Butt Diameter, in.	13 1/2
Length, ft	52
Penetration Depth, ft	41
0-13	WOH
14	2
15	2
16	1
17	1
18	WOH
19	
20	
21	
22	
23	↓

File No.	TP-1
24	WOH
25	
26	
27	
28	
29	↓
30	2
31	2
32	2
33	2
34	2
35	2
36	2
37	2
38	2
39	3
40	20
41	25 @ 4"

WOH – weight of hammer

LOADING SCHEDULE AND RECORD OF SETTLEMENT

TEST PILE NO: TP-1

DATE TESTED: July 25, 2012

TIME HOURS	ELAPSED TIME, MIN	LOAD IN TONS	SETTLEMENT IN INCHES		REMARKS
			ENGINEER'S LEVEL	PIANO WIRE	
9:00 am		1.5			Increment #1
9:01 am	1		0.01	0.01	
9:03 am	3		0.01	0.01	
9:05 am	5		0.01	0.01	
		3			Increment #2
9:06 am	1		0.02	0.01	
9:08 am	3		0.02	0.01	
9:10 am	5		0.02	0.01	
		4.5			Increment #3
9:11 am	1		0.04	0.03	
9:13 am	3		0.04	0.03	
9:15 am	5		0.04	0.03	
		6			Increment #4
9:16 am	1		0.05	0.04	
9:18 am	3		0.05	0.04	
9:20 am	5		0.05	0.04	
		7.5			Increment #5
9:21 am	1		0.06	0.04	
9:23 am	3		0.06	0.04	
9:25 am	5		0.06	0.04	
		9			Increment #6
9:26 am	1		0.07	0.05	
9:28 am	3		0.07	0.05	
9:30 am	5		0.07	0.05	
		10.5			Increment #7
9:31 am	1		0.08	0.07	
9:33 am	3		0.08	0.07	
9:35 am	5		0.08	0.07	
		12			Increment #8
9:36 am	1		0.10	0.08	
9:38 am	3		0.10	0.08	
9:40 am	5		0.10	0.08	

LOADING SCHEDULE AND RECORD OF SETTLEMENT (Continued)

TIME HOURS	ELAPSED TIME, MIN	LOAD IN TONS	SETTLEMENT IN INCHES		REMARKS
			ENGINEER'S LEVEL	PIANO WIRE	
		13.5			Increment #9
9:41 am	1		0.11	0.09	
9:43 am	3		0.11	0.09	
9:45 am	5		0.11	0.09	
		15			Increment #10
9:46 am	1		0.12	0.10	
9:48 am	3		0.12	0.10	
9:50 am	5		0.12	0.10	
		16.5			Increment #11
9:51 am	1		0.13	0.11	
9:53 am	3		0.13	0.11	
9:55 am	5		0.13	0.11	
		18			Increment #12
9:56 am	1		0.15	0.13	
9:58 am	3		0.15	0.13	
10:00 am	5		0.15	0.13	
		19.5			Increment #13
10:01 am	1		0.16	0.14	
10:03 am	3		0.16	0.14	
10:05 am	5		0.16	0.14	
		21			Increment #14
10:06 am	1		0.18	0.16	
10:08 am	3		0.18	0.16	
10:10 am	5		0.18	0.17	
		22.5			Increment #15
10:11 am	1		0.21	0.18	
10:13 am	3		0.21	0.18	
10:15 am	5		0.21	0.18	
		24			Increment #16
10:16 am	1		0.23	0.20	
10:18 am	3		0.23	0.20	
10:20 am	5		0.23	0.20	
		25.5			Increment #17
10:21 am	1		0.24	0.22	
10:23 am	3		0.24	0.22	
10:25 am	5		0.24	0.22	

LOADING SCHEDULE AND RECORD OF SETTLEMENT (Continued)

TIME HOURS	ELAPSED TIME, MIN	LOAD IN TONS	SETTLEMENT IN INCHES		REMARKS
			ENGINEER'S LEVEL	PIANO WIRE	
		27			Increment #18
10:26 am	1		0.26	0.22	
10:28 am	3		0.26	0.22	
10:30 am	5		0.26	0.22	
		28.5			Increment #19
10:31 am	1		0.28	0.25	
10:33 am	3		0.28	0.27	
10:35 am	5		0.29	0.27	
		30			Increment #20
10:36 am	1		0.30	0.29	
10:38 am	3		0.31	0.29	
10:40 am	5		0.31	0.29	
		31.5			Increment #21
10:41 am	1		0.34	0.31	
10:43 am	3		0.34	0.31	
10:45 am	5		0.34	0.31	
		33			Increment #22
10:46 am	1		0.35	0.33	
10:48 am	3		0.35	0.33	
10:50 am	5		0.35	0.33	
		34.5			Increment #23
10:51 am	1		0.38	0.36	
10:53 am	3		0.38	0.36	
10:55 am	5		0.38	0.36	
		36			Increment #24
10:56 am	1		0.40	0.38	
10:58 am	3		0.40	0.38	
11:00 am	5		0.41	0.38	
		37.5			Increment #25
11:01 am	1		0.42	0.40	
11:03 am	3		0.42	0.40	
11:05 am	5		0.42	0.40	
		39			Increment #26
11:06 am	1		0.45	0.42	
11:08 am	3		0.45	0.43	
11:10 am	5		0.45	0.43	

LOADING SCHEDULE AND RECORD OF SETTLEMENT (Continued)

TIME HOURS	ELAPSED TIME, MIN	LOAD IN TONS	SETTLEMENT IN INCHES		REMARKS
			ENGINEER'S LEVEL	PIANO WIRE	
		40.5			Increment #27
11:11 am	1		0.47	0.45	
11:13 am	3		0.48	0.45	
11:15 am	5		0.48	0.45	
		42			Increment #28
11:16 am	1		0.51	0.47	
11:18 am	3		0.51	0.47	
11:20 am	5		0.51	0.48	
		43.5			Increment #22
11:21 am	1		0.53	0.50	
11:53 am	3		0.53	0.51	
11:25 am	5		0.53	0.51	
		45			Increment #23
11:26 am	1		0.54	0.52	
11:28 am	3		0.55	0.52	
11:30 am	5		0.55	0.52	
		46.5			Increment #24
11:31 am	1		0.57	0.53	
11:33 am	3		0.57	0.53	
11:35 am	5		0.57	0.53	
		48			Increment #25
11:36 am	1		0.61	0.57	
11:38 am	3		0.61	0.57	
11:40 am	5		0.61	0.58	
		49.5			Increment #26
11:41 am	1		0.62	0.60	
11:43 am	3		0.62	0.60	
11:45 am	5		0.62	0.60	
		50			Increment #26
11:46 am	1		0.64	0.61	
11:48 am	3		0.64	0.61	
11:50 am	5		0.64	0.61	

LOADING SCHEDULE AND RECORD OF SETTLEMENT (Continued)

TIME HOURS	ELAPSED TIME, MIN	LOAD IN TONS	SETTLEMENT IN INCHES		REMARKS
			ENGINEER'S LEVEL	PIANO WIRE	
		37.5			Decrement #1
11:51 am	1		0.57	0.55	
11:53 am	3		0.57	0.55	
11:55 am	5		0.57	0.55	
		25.5			Decrement #2
11:56 am	1		0.47	0.44	
11:58 am	3		0.47	0.44	
12:00 pm	5		0.47	0.43	
		12			Decrement #3
12:01 pm	1		0.33	0.30	
12:03 pm	3		0.33	0.30	
12:05 pm	5		0.33	0.30	
		0			Decrement #4
12:06 pm	1		0.18	0.16	
12:08 pm	3		0.18	0.16	
12:10 pm	5		0.18	0.15	
12:40 pm	30		0.18		Held Zero 30 Min.

SUMMARY AND CONCLUSION

The test pile was driven on July 11, 2012 and loaded on July 25, 2012 after a fourteen (14) day waiting period. The pile was installed within an 18 inch diameter casing reamed out to about 13 feet to disengage friction in the upper 13 feet of the pile and simulate the loading condition exerted on the pile. The test pile was located at the northeast corner of the site near the intersection of Iberville and Rampart Street.


The large timber test pile TP-1 was loaded in 1.5 ton increments up to 50 tons. The pile maintained each load increment up to 50 tons with little or no movement after the initial application of each load increment. At 50 tons, a total of 0.64 inch of settlement was recorded. The test load was then removed in four equal decrements and the settlement was monitored for 5 minutes. The net settlement measured after the rebound period of 30 minutes was 0.18 inch.

Based on the load test results, the large timber pile meets the allowable design load capacity of 12 tons. If you should have any questions, please do not hesitate to call.

Respectfully Submitted,
Stratum Engineering, LLC

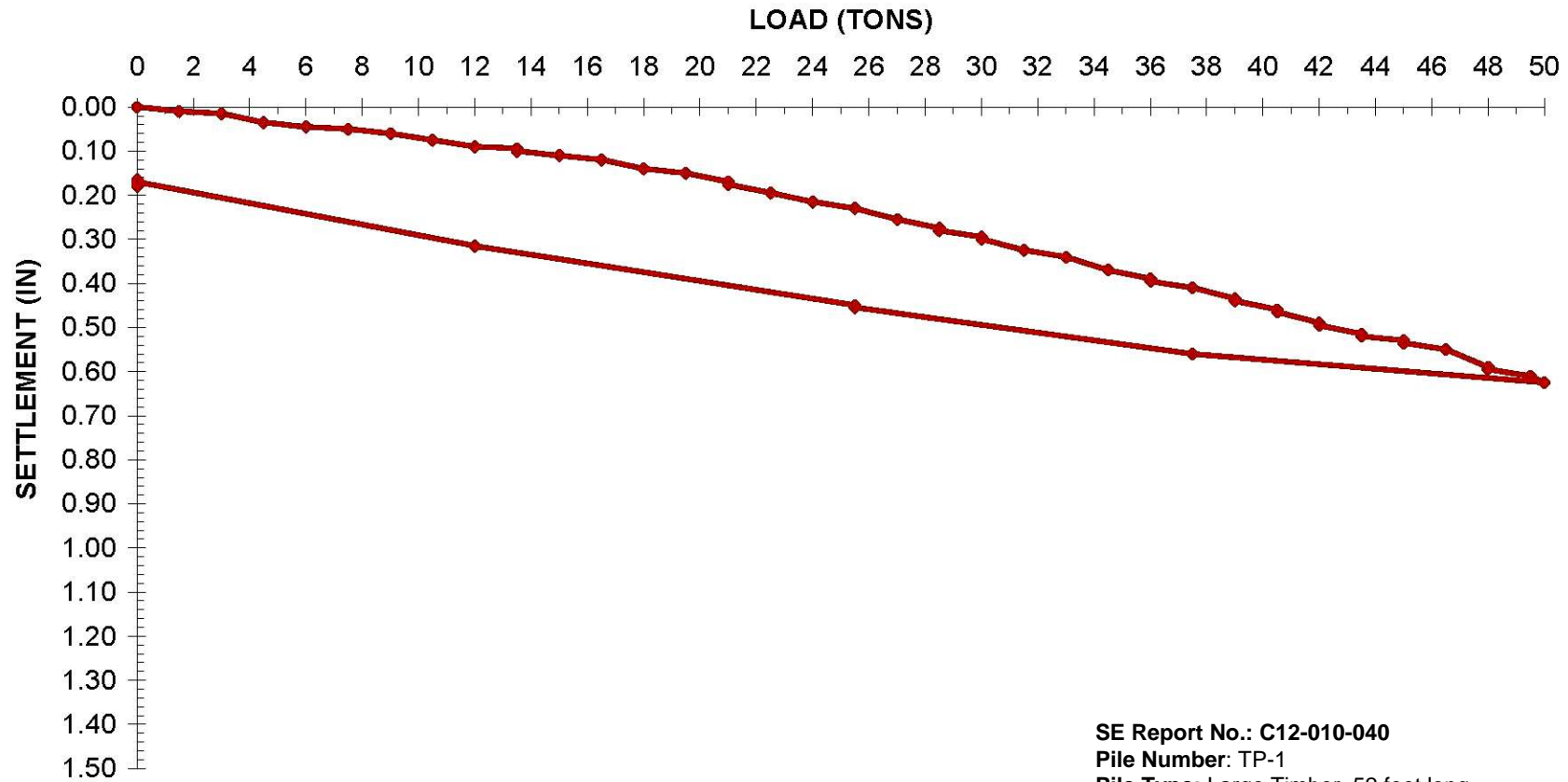


Ryan Kaufmann
Project Manager



Tony Maroun, P.E.
Principal

APPENDIX



SE Report No.: C12-010-040
Pile Number: TP-1
Pile Type: Large Timber, 52 feet long
Tip Diameter: 7 inch
Butt Diameter: 13 ½ inch
Penetration Depth: 41 feet
Casing Reamed Out Depth: 13 feet
Date Driven: July 11, 2012
Test Date: July 25, 2012



STRATUM ENGINEERING, LLC
148 W HOWZE BEACH ROAD, SLIDELL, LOUISIANA

VIBRATION / ACOUSTICAL MONITORING REPORT

TESTED FOR: Broadmoor, LLC	PROJECT: Saenger Theater Redevelopment-2012	SCALE: 0 TO 2.5 IPS
DATE: 7-10-2012	MONITOR NUMBER: 4866	
TECHNICIAN: David Ziegler	ACOUSTICAL INSTRUMENTS: MS-2G	WO No. 5043

TIME		LOCATION #	DISTANCE FROM SOURCE	MAXIMUM PPV (IN/SEC)	DESCRIPTION OF WORK
7:18 AM	8:00 AM	1	50 ft. to 75 ft.	0.045	Driving 70 ft. H-pile (reaction pile).
8:08 AM	10:15 AM	1	50 ft. to 75 ft.	0.085	Driving 70 ft. H-pile (reaction pile).

VIBRATION SOURCE: MKT V-20 Vibratory Hammer
NOTIFICATION: No readings in excess of 0.25 IPS were recorded on this date.
MONITOR LOCATION: 1) Intersection of Basin St. and Iberville St. (New Orleans, LA)
REMARKS:



STRATUM ENGINEERING, LLC
148 W HOWZE BEACH ROAD, SLIDELL, LOUISIANA

VIBRATION / ACOUSTICAL MONITORING REPORT

TESTED FOR: Broadmoor, LLC	PROJECT: Saenger Theater Redevelopment-2012	SCALE: 0 TO 2.5 IPS
DATE: 7-11-2012	MONITOR NUMBER: 4866	
TECHNICIAN: Sandee Frickey	ACOUSTICAL INSTRUMENTS: MS-2G	WO No. 5055

TIME		LOCATION #	DISTANCE FROM SOURCE	MAXIMUM PPV (IN/SEC)	DESCRIPTION OF WORK
6:57 AM	1:20 PM	1	70 ft. to 80 ft.	0.440	Driving 52 ft. large timber test pile.
1:32 PM	4:19 PM	2	60 ft. to 70 ft.	0.060	Driving 52 ft. large timber test pile.

VIBRATION SOURCE: Vulcan 01 hammer - generating 15,000 ft.-lbs. of energy per blow.

NOTIFICATION: Contractor was notified of all readings in excess of 0.25 IPS recorded on this date.

MONITOR LOCATION: 1) NW corner of building at 1101-1199 Iberville St.

REMARKS: Crane moving matts cause the high reading of 0.44 IPS at 9:52am.

Beerman Precision, Inc.
4206 Howard Avenue, New Orleans, LA 70125
504-207-6023 Fax: 504-207-6044

LOAD CELL CALIBRATION REPORT

CUSTOMER: <u>STRATUM ENGINEERING</u>	PROJECT: <u>PILE TEST</u>
ORDER NO: <u>6230</u>	DATE: <u>July 24, 2012</u>
<hr/>	
LOAD CELL <u>MEA3000-300-3, 150 TON C/H</u>	SERIAL NO: <u>09-126</u>
INDICATOR <u>VISHAY P3 DIGITAL READOUT</u>	SERIAL NO: <u>191221</u>

This report covers the results of Calibration for the above Load Cell. Calibration was performed utilizing our 500-Ton Master Load Cell, S/N 9403, with the digital readout indicator, S/N 189935. The Master Load Cell was calibrated on MARCH 22, 2012 in accordance with ASTM E4 and to be within a 1% tolerance. Results of your current calibration are shown on the following pages:

Temperature during test 82 Degrees Fahrenheit

READOUT INDICATOR INITIAL SETTINGS		
AMP ZERO:	<input type="text" value="N/A"/>	+/-0002
FS Mv/V (GF):	<input type="text" value="1.300"/>	+/-0001
BALANCE:	<input type="text" value="0.0"/>	+/-0002
MATCH THE FOLLOWING TO INDICATOR:		
LC WIRE COLOR	POST	
BLK/GRN WIRE	<input type="text" value="S-"/>	
BLACK WIRE	<input type="text" value="P-"/>	
WHITE WIRE	<input type="text" value="S+"/>	
RED WIRE	<input type="text" value="P+"/>	

IMPORTANT
CONNECT ALL WIRES TO TERMINAL POST LISTED IN INITIAL SETTINGS
TURN ON POWER. LET LOAD CELL WARM UP FOR 15-20 MINUTES.
CHECK ALL INITIAL SETTINGS BY PRESSING APPROPRIATE BUTTON. RESET TO SETTINGS LISTED IF NOT WITHIN TOLERANCE.
READ ATTACHED INSTRUCTIONS AND REVIEW HYDRAULIC SCHEMATIC

By:



Date:

July 24, 2012

BEERMAN PRECISION, INC.

Beerman Precision, Inc.

4206 Howard Avenue, New Orleans, LA 70125
504-207-6023 Fax: 504-207-6044

LOAD CELL CALIBRATION REPORT

CUSTOMER: STRATUM ENGINEERING

PROJECT: PILE TEST

CONTRACT NO: 6230

DATE: July 24, 2012

LOAD CELL: MEA3000-300-3, 150 TON C/H

SERIAL NO: 09-126

INDICATOR: VISHAY P3 DIGITAL READOUT

SERIAL NO: 191221

STANDARD LOAD (TONS)	SEPARATE TEST RUNS			AVE OUTPUT OF LOAD CELL
	1ST RUN	2ND RUN	3RD RUN	
0.0	0	0	0	0
1.5	1.5	1.4	1.5	1.5
3.0	3.0	3.0	3.1	3.0
4.5	4.6	4.6	4.6	4.6
6.0	6.1	6.2	6.2	6.2
7.5	7.6	7.7	7.7	7.7
9.0	9.1	9.3	9.3	9.2
10.5	10.7	10.7	10.8	10.7
12.0	12.2	12.5	12.3	12.3
13.5	13.8	13.9	13.9	13.9
15.0	15.3	15.5	15.4	15.4
16.5	16.8	17.0	17.0	16.9
18.0	18.3	18.6	18.6	18.5
19.5	19.9	20.0	20.0	20.0
21.0	21.5	21.6	21.5	21.5
22.5	23.0	23.2	23.1	23.1
24.0	24.5	24.7	24.7	24.6
25.5	26.1	26.1	26.2	26.1
27.0	27.6	27.7	27.7	27.7
28.5	29.1	29.2	29.2	29.2
30.0	30.6	30.7	30.7	30.7
31.5	32.1	32.2	32.3	32.2
33.0	33.6	33.7	33.8	33.7
34.5	35.1	35.2	35.2	35.2
36.0	36.7	36.7	36.7	36.7
37.5	38.2	38.2	38.2	38.2
39.0	39.7	39.7	39.7	39.7
40.5	41.2	41.1	41.2	41.2
42.0	42.7	42.6	42.7	42.7
43.5	44.2	44.1	44.2	44.2
45.0	45.6	45.6	45.7	45.6

BY: 

DATE: July 24, 2012

4206 Howard Avenue, New Orleans, LA 70125
504-207-6023 Fax: 504-207-6044

CUSTOMER: STRATUM ENGINEERING

PROJECT: PILE TEST

REPORT NO: 6230

DATE: July 24, 2012

CYLINDER: MEA3000-300-3, 150 TON C/H

SERIAL NO: 09-126

GAUGE: VISHAY P3 DIGITAL READOUT

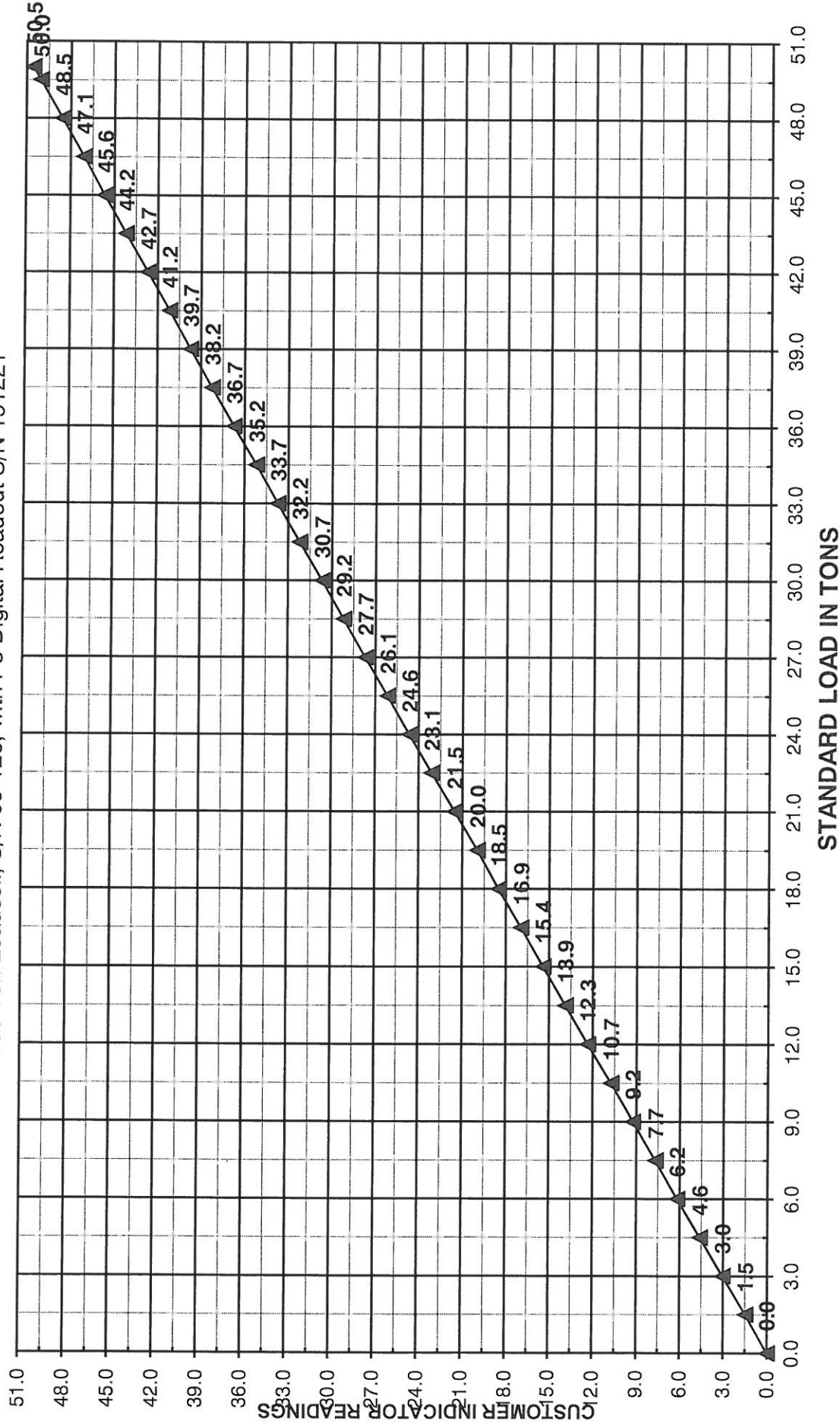
SERIAL NO: 191221

[illegible]

Beerman Precision, Inc.
 4206 HOWARD AVENUE NEW ORLEANS, LA 70125

504 007 0000 FAX 504 007 0044

150 Ton Loadcell, S/N 09-126, with P3 Digital Readout S/N 191221



Beerman Precision, Inc.
4206 Howard Avenue, New Orleans, LA 70125
504-207-6023 Fax: 504-207-6070

JACK CALIBRATION REPORT

CUSTOMER: STRATUM ENGINEERING

PROJECT: PILE TEST

CONTRACT NO: 6230

DATE: July 24, 2012

CYLINDER: RC756

SERIAL NO: 1907

GAUGE: 6IN. WIKA, 10,000 PSI GAUGE

SERIAL NO: Z7006JF

This report covers the results of Calibration for the above Hydraulic Cylinder. Calibration was performed utilizing our 500-Ton Master Load Cell, S/N 9403, with the digital readout indicator, S/N 189935. The Master Load Cell was recently calibrated on March 22, 2012 in accordance with ASTM E4 and is within a 1% tolerance. Results of current calibration are shown on the following pages:

Temperature during test 82 Degrees Fahrenheit

By: 

Date: July 24, 2012

Beerman Precision, Inc.

4206 Howard Avenue, New Orleans, LA 70125
504-207-6023 Fax: 504-207-6070

PRESSURE GAUGE CERTIFICATION

CUSTOMER: STRATUM ENGINEERING

PROJECT: PILE TEST

CONTRACT NO: 6230

DATE: July 24, 2012

GAUGE: 6IN. WIKA; 10,000 PSI GAUGE

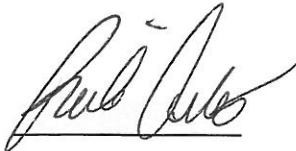
SERIAL NO: Z7006JF

We certify the above hydraulic pressure gauge has been tested against our primary standard, an Amthor Dead Weight Tester and found to be within an accuracy of $\pm 1/2\%$ of full scale. The Amthor Tester was last certified on May 29, 2012 to 0.1% accuracy and traceable to the National Institute of Standards and Technology (NIST).

Standard Pressure (PSI) Your Pressure Gauge (PSI)

0	0
1000	1050
2000	2050
3000	3050
4000	4050
5000	5050
6000	6050
7000	7050
8000	8050
9000	9050
10,000	10,000

By:



Date:

July 24, 2012

BEERMAN PRECISION, INC.

Beerman Precision, Inc.
4206 Howard Avenue New Orleans, LA 70125
504-207-6023 Fax: 504-207-6070

JACK CALIBRATION REPORT

CUSTOMER: STRATUM ENGINEERING

PROJECT: PILE TEST

CONTRACT NO: 6230

DATE: July 24, 2012

CYLINDER: RC756

SERIAL NO: 1907

GAUGE: 6IN. WIKA; 10,000 PSI GAUGE

SERIAL NO: Z7006JF

STANDARD LOAD (TONS)	GAUGE READINGS IN PSI AT PISTON EXTENSION OF						AVE GAUGE PRESSURE
	1	INCHES	3	INCHES	5	INCHES	
1.5	300		325		350		325
3.0	500		550		575		542
4.5	725		750		775		750
6.0	925		975		1000		967
7.5	1125		1200		1225		1183
9.0	1350		1400		1450		1400
10.5	1550		1600		1675		1608
12.0	1750		1825		1900		1825
13.5	1950		2050		2125		2042
15.0	2175		2250		2350		2258
16.5	2400		2475		2550		2475
18.0	2600		2700		2775		2692
19.5	2800		2925		3000		2908
21.0	3000		3150		3200		3117
22.5	3200		3375		3425		3333
24.0	3400		3600		3650		3550
25.5	3625		3800		3875		3767
27.0	3850		4025		4075		3983
28.5	4050		4250		4300		4200
30.0	4275		4450		4525		4417
31.5	4475		4675		4750		4633
33.0	4675		4900		4950		4842
34.5	4875		5100		5175		5050
36.0	5075		5325		5400		5267
37.5	5275		5550		5600		5475
39.0	5500		5750		5800		5683
40.5	5700		5800		6000		5833
42.0	5900		6200		6225		6108
43.5	6100		6400		6425		6308
45.0	6325		6600		6650		6525
46.5	6550		6800		6850		6733

BY: 

DATE: July 24, 2012

JACK CALIBRATION REPORT(CONT'D)

PROJECT: PILE TEST

DATE: July 24, 2012

SERIAL NO: 1907

SERIAL NO: Z7006JF

[illegible]

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